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PRODUCT APPROVAL SUPPORTING CALCULATIONS

F-4500 Inswing and Outswing Doors (non-Impact)

REPORT TO:

JELD-WEN WINDOWS & DOORS 3737 LAKEPORT BLVD KLAMATH FALLS, OREGON

REPORT NUMBER: NCTL-110-26137-1 REPORT DATE: 04/02/23

This item has been digitally signed and sealed by Joseph A. Reed, PE on the date adjacent to the seal.

Joseph A. Reed, PE FL PE 58920 FL REG 33474

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Scope

National Certified Testing Laboratories was contracted by Jeld-Wen Windows & Doors to evaluate alternate installation methods for their F-4500 Inswing and Outswing Doors (non-Impact). The evaluation is based on physical testing and product certifications. Reference standards utilized in this project include:

Florida Building Code, Building. International Code Council.

ANSI/AWC *National Design Specification (NDS) for Wood Construction*. American Wood Council.

AISI S100 North American Specification for the Design of Cold-Formed Steel Structural Members. American Iron and Steel Institute.

ICC-ES Report ESR-1976 *ITW Buildex TEKS Self-Drilling Fasteners*. ICC Evaluation Service.

NOA 21-0201.06 Tapcon Concrete and Masonry Anchors with Advanced Threadform Technology. Miami-Dade County Product Control Section.

The anchorage analyses presented herein do not address the water resistance, water penetration or air infiltration performance of the installation method or the installed product. In addition, the analyses rely on the assumption that the building substrate is capable of withstanding incurred loads.

Certification of Independence

In accordance with Rule 61G20-3 Florida Administrative Code, National Certified Testing Laboratories hereby certifies the following:

- National Certified Testing Laboratories does not have, nor does it intend to acquire
 or will it acquire, a financial interest in any company manufacturing or distributing
 products tested or labeled by the agency.
- National Certified Testing Laboratories is not owned, operated or controlled by any company manufacturing or distributing products it tests or labels.
- Joseph A. Reed, P.E. does not have nor will acquire, a financial interest in any company manufacturing or distributing products for which the reports are being issued.
- Joseph A. Reed, P.E does not have, nor will acquire, a financial interest in any other entity involved in the approval process of the product.



Analyses

Summary of Test Results

The following table summarizes the various F-4500 Inswing and Outswing Doors (non-Impact) products and their corresponding performance levels which have been established by testing or product certification.

Table 1 Summary of Test Results

Series/Model	Test Report Number	Size (W x H)	Performance
F-4500 Inswing Door (XX) Through Frame Install	NCTL-310-22-108 (Rev. 2, 03/02/23)	72" x 79"	+50/-55 psf
F-4500 Outswing Door (XX) Through Frame Install	NCTL-310-23-002 (Rev. 4, 03/14/23)	72" x 78"	+50/-55 psf

Testing documented in Table 1 was conducted by National Certified Testing Laboratories in Everett, Washington (Florida Department of Business & Professional Regulation Test Lab No. TST9341, A2LA Laboratory Certificate 3054.03).

As-Tested Installation Analysis

For air/water/structural testing the test specimen was secured to a 2x Spruce-Pine-Fir buck. The as-tested installation method is evaluated on page 3 to page 5. These capacities will be used to prove acceptable alternate anchors and substrates for the product.

Alternate Anchorages

Calculations on page 6 through page 15 determine the design capacity of alternate installation anchorages for the product.

Anchorage Requirements

As-tested spacing must be maintained. It must be determined the anchorages are not overloaded for the approved product size and design pressures. Calculations presented on page 16 show the alternate anchorages are acceptable for the established product performance.

Anchorage requirements established by this report are accurately presented in Drawing D1000255.

Attachments

Appendix A – Revision Log (1 page)



<u>As-Tested Installation – Through Frame to Wood</u>

#8 Flat Head Screw; 1-1/2" penetration to wood

3/4" thick Wood Frame (G = 0.42)

1/4" Maximum Shim Space

Spruce-Pine-Fir 2x Wood Substrate Minimum (G=0.42)

Allowable Shear of #8 Flat Head Screw

Z' = 110 lb (See Following 2 Pages)

Bending of #8 Flat Head Screw

L = 1/4" (maximum shim space)

 $S = \pi d^3/32 = \pi (0.131)^3/32 = 0.000221 in^3$

 $F_b = (1.3)(0.6F_y) = (1.3)(0.6)(90,000 \text{ psi}) = 70,200 \text{ psi} (1.3 \text{ weak axis factor})$

 $F_b = M/S = (VL/2)/S (L/2 \text{ for guided bending})$

 $V = 2SF_b/L = (2)(0.000221 \text{ in}^3)(70,200 \text{ psi})/0.25" = 124 \text{ lb}.$

Capacity of Connection is 110 lb



As-Tested Installation – Through Frame to Wood (Continued)

Lateral Design Strength of Wood Connections

Data

Fastener				
Fastener	=	#8 Wood Screw		
Shank Dia	=	0.164	in.	
Root Dia.	=	0.131	in.	
F_{yb}	=	90,000	psi	
Fastener length	=	2.000	in.	
F_{yb}	= = =	90,000	psi	

Main Member

Material	=		SPF	
G	=	0.42		
θ	=	90	<= (Angle of loa	d to grain $0^{\circ} \le \theta \le 90^{\circ}$)
F_{e}	=	3,350	psi	
Thickness	=	1.500	in.	

Side Member

Material	=		SPF	
G	=	0.42		
θ	=	90	<= (Angle of loa	d to grain $0^{\circ} \le \theta \le 90^{\circ}$)
F_{es}	=	3,350	psi	
Thickness	=	0.750	in.	

Calculations

Lateral Bearing Factors

D	=	0.131	in.
$\ell_{\rm m}$	=	0.922	in.
$K_{\boldsymbol{\theta}}$	=	1.25	
K_D	=	2.20	
R_{e}	=	1.000	
R_{t}	=	1.23	
\mathbf{k}_1	=	0.4659	
k_2	=	1.2549	
k_3	=	1.37	

<===== Minimum Value



As-Tested Installation - Through Frame to Wood (Continued)

Yield Mode	R_d
$I_{\rm m}$, $I_{\rm s}$	2.20
II	2.20
III _m , III _s , IV	2.20

Lateral Design Values, Z

$Mode \ I_m$	=	184	lbf
Mode I _s	=	150	lbf
Mode II	=	70	lbf
$Mode III_m$	=	77	lbf
$Mode III_s$	=	69	lbf
Mode IV	=	78	lbf
C_{D}	=	1.6	

Wet Service Factor

Fabrication/In-Service Dry/Dry

 C_{M} 1.0

In service temperature T≤100°F

> C_{t} = 1.0 C_g 1.0 =

 $\boldsymbol{C}_{\!\Delta}$ 1.0 =

Is fastener installed in end grain? No

> 1.00 No

Is fastener part of a diaphragm?

1.0

 C_{di} Is fastener toe-nailed? No

> 1.00 C_{tn}

Z' <u>110</u> lbf



<u>Alternate Installation - Strap Anchor to Wood</u>

#8 x 1-1/2" Flat Head Screw securing strap to substrate

Spruce-Pine-Fir 2x Wood Substrate Minimum (G=0.42)

Two #8 Screws securing strap to frame

3/4" thick Wood Frame

20 gauge (0.033" thick) 33 KSI Steel Strap Anchor

1/4" Maximum Shim Space

Allowable Shear of #8 x 1-1/2" Flat Head Screw

Z' = 122 lb (See Following 2 Pages)

Bending of #8 x 1-1/2" Flat Head Screw

L = 1/4" (maximum shim space)

 $S = \pi d^3/32 = \pi (0.131)^3/32 = 0.000221 \text{ in}^3$

 $F_b = (1.3)(0.6F_y) = (1.3)(0.6)(90,000 \text{ psi}) = 70,200 \text{ psi} (1.3 \text{ weak axis factor})$

 $F_b = M/S = (VL/2)/S (L/2 \text{ for guided bending})$

 $V = 2SF_b/L = (2)(0.000221 \text{ in}^3)(70,200 \text{ psi})/0.25" = 124 \text{ lb}.$

Bearing of #8 Screw on Frame

$$V_a = F_eDt/K_D = (3,350 \text{ psi})(0.164")(0.75")/(10(0.164)+0.5) = 193 \text{ lb}$$

Bearing of #8 Screw on Strap Anchor

 $V_a = 2.7 Dt F_{tu}/3.0$

 $V_a = 2.7(0.164")(0.033")(45,000 psi)/3.0$

 $V_a = 219 \text{ lb.}$

Capacity of Connection is 122 lb



<u>Alternate Installation – Strap Anchor to Wood</u> (Continued)

Lateral Design Strength of Wood Connections

Data

Fastener				
Fastener	=	#8 W	#8 Wood Screw	
Shank Dia	=	0.164	in.	
Root Dia.	=	0.131	in.	
F_{yb}	=	90,000	psi	
Fastener length	=	2.500	in.	
Main Member				
Matarial	_		CDE	

Material	=		SPF	
G	=	0.42		
θ	=	90	<= (Angle of loa	d to grain $0^{\circ} \le \theta \le 90^{\circ}$)
F_{e}	=	3,350	psi	
Thickness	=	1.500	in.	

Side Member

Material	=	ASTM A 653	<mark>3, Grade 33 Steel</mark>
G	=	N/A	
θ	=	90	$<=$ (Angle of load to grain $0^{\circ} < \theta < 90^{\circ}$)
F_{es}	=	61,850	psi
Thickness	=	0.033	in.

Calculations

Lateral Bearing Factors

	0		
D	=	0.131	in.
$\ell_{\rm m}$	=	1.500	in.
$K_{\boldsymbol{\theta}}$	=	1.25	
K_D	=	2.20	
R_{e}	=	0.054	
R_{t}	=	45.45	
\mathbf{k}_1	=	1.0041	
k_2	=	0.5032	
k_3	=	23.87	

Yield Mode	R_d
$I_{\rm m}$, $I_{\rm s}$	2.20
II	2.20
III _m , III _s , IV	2.20



Alternate Installation - Strap Anchor to Wood (Continued)

Lateral Des	ign Valu	_		
$Mode I_m$	=	299	lbf	
Mode I _s	=	122	lbf	
Mode II	=	122	lbf	
$Mode\:III_{m}$	=	136	lbf	
Mode III _s	=	77	lbf	<===== Minimum Value
Mode IV	=	108	lbf	
C_D	=	1.6		
Wet Service Factor				
Fabrication/In-	Service	Dry/Dry		
C_{M}	=	1.0		
In service temperature		T:	≤100°F	
C_{t}	=	1.0		
C_g		1.0		
${f C}_{\Delta}$	=	1.0		
Is fastener installed in end grain?		No		
C_{eg}	=	1.00		
Is fastener part of a diaphragm?		No		
C_{di}	=	1.0		
Is fastener toe-nailed?		No		
C_{tn}	=	1.00		
Z'	=	<u>122</u>	lbf	



Alternate Installation – Through Frame to Concrete

3/16" Tapcon Anchor

2-1/2" Minimum Edge Distance, 1-1/4" Minimum Embedment

1/4" Maximum Shim Space

Minimum f'c = 3,000 psi Concrete

Allowable Shear of 3/16" Tapcon Anchor

 $P_{ss}/\Omega = 181 \text{ lb}$ (NOA 21-0201.06)

Bearing of 3/16" Tapcon Anchor on Frame

 $V_a = F_eDt/K_D = (3,350 \text{ psi})(0.170")(0.75")/(10(0.170)+0.5) = 194 \text{ lb}$

Bending of 3/16" Tapcon Anchor

L = 1/4" (Maximum Shim Space)

 $S = \pi d^3/32 = \pi (0.170")^3/32 = 0.000482 \text{ in}^3$

 $F_b = (1.3)(0.6F_y) = (1.3)(0.6)(137,000 \text{ psi}) = 106,860 \text{ psi} (1.3 \text{ weak axis factor})$

 $F_b = M/S = (VL/2)/S (L/2 \text{ for guided bending})$

 $V = 2SF_b/L = (2)(0.000482 \text{ in}^3)(106,860 \text{ psi})/0.25" = 412 \text{ lb}.$

Capacity of Connection is 181 lb



Alternate Installation – Through Frame to CMU

3/16" Tapcon Anchor

2-1/2" Minimum Edge Distance, 1-1/4" Minimum Embedment

1/4" Maximum Shim Space

Minimum ASTM C90 Concrete Masonry Unit

Allowable Shear of 3/16" Tapcon Anchor

 $P_{ss}/\Omega = 135 \text{ lb}$ (NOA 21-0201.06)

Bearing of 3/16" Tapcon Anchor on Frame

 $V_a = F_eDt/K_D = (3,350 \text{ psi})(0.170")(0.75")/(10(0.170)+0.5) = 194 \text{ lb}$

Bending of 3/16" Tapcon Anchor

L = 1/4" (Maximum Shim Space)

 $S = \pi d^3/32 = \pi (0.170")^3/32 = 0.000482 in^3$

 $F_b = (1.3)(0.6F_y) = (1.3)(0.6)(137,000 \text{ psi}) = 106,860 \text{ psi} (1.3 \text{ for weak axis bending})$

 $F_b = M/S = (VL/2)/S (L/2 \text{ for guided bending})$

 $V = 2SF_b/L = (2)(0.000482 \text{ in}^3)(106,860 \text{ psi})/0.25" = 412 \text{ lb}.$

Capacity of Connection is 135 lb



Alternate Installation – Strap Anchor to Concrete

3/16" Tapcon Anchor; 2-1/2" Minimum Edge Distance, 1-1/4" Minimum Embedment

#8 Screws Connecting Strap to Frame

3/4" thick Wood Frame

20 gauge (0.033" thick) 33 KSI Steel Strap Anchor

1/4" Maximum Shim Space

Minimum f'_c = 3,000 psi Concrete

Allowable Shear of 3/16" Tapcon Anchor

 $P_{ss}/\Omega = 181 \text{ lb}$ (NOA 21-0201.06)

Bearing of 3/16" Tapcon Anchor on Strap Anchor

 $V_a = 2.7 Dt F_{tu}/3.0$

 $V_a = 2.7(0.170")(0.033")(45,000 psi)/3.0$

 $V_a = 227 \text{ lb.}$

Bending of 3/16" Tapcon Anchor

L = 1/4" (Maximum Shim Space)

 $S = \pi d^3/32 = \pi (0.170'')^3/32 = 0.000482 in^3$

 $F_b = (1.3)(0.6F_v) = (1.3)(0.6)(137,000 \text{ psi}) = 106,860 \text{ psi} (1.3 \text{ weak axis factor})$

 $F_b = M/S = (VL/2)/S (L/2 \text{ for guided bending})$

 $V = 2SF_b/L = (2)(0.000482 \text{ in}^3)(106,860 \text{ psi})/0.25" = 412 \text{ lb}.$

Bearing of #8 Screw on Strap Anchor

 $V_a = 2.7 Dt F_{tu}/3.0$

 $V_a = 2.7(0.164")(0.033")(45,000 psi)/3.0$

 $V_a = 219 \text{ lb.}$



<u>Alternate Installation – Strap Anchor to Concrete</u> (Continued)

Bearing of #8 Screw on Frame

$$V_a = F_eDt/K_D = (3,350 \text{ psi})(0.164")(0.75")/(10(0.164)+0.5) = 193 \text{ lb}$$

Capacity of Connection is 181 lb (only one concrete screw per strap)



Alternate Installation - Strap Anchor to CMU

3/16" Tapcon Anchor; 2-1/2" Minimum Edge Distance, 1-1/4" Minimum Embedment

#8 Screws Connecting Strap to Frame

3/4" thick Wood Frame

20 gauge (0.033" thick) 33 KSI Steel Strap Anchor

1/4" Maximum Shim Space

Minimum ASTM C90 Concrete Masonry Unit

Allowable Shear of 3/16" Tapcon Anchor

 $P_{ss}/\Omega = 135 \text{ lb}$ (NOA 21-0201.06)

Bearing of 3/16" Tapcon Anchor on Strap Anchor

 $V_a = 2.7 Dt F_{tu}/3.0$

 $V_a = 2.7(0.170")(0.033")(45,000 psi)/3.0$

 $V_a = 227 \text{ lb.}$

Bending of 3/16" Tapcon Anchor

L = 1/4" (Maximum Shim Space)

 $S = \pi d^3/32 = \pi (0.170'')^3/32 = 0.000482 in^3$

 $F_b = (1.3)(0.6F_v) = (1.3)(0.6)(137,000 \text{ psi}) = 106,860 \text{ psi} (1.3 \text{ for weak axis bending})$

 $F_b = M/S = (VL/2)/S (L/2 \text{ for guided bending})$

 $V = 2SF_b/L = (2)(0.000482 \text{ in}^3)(106,860 \text{ psi})/0.25" = 412 \text{ lb}.$

Bearing of #8 Screw on Strap Anchor

 $V_a = 2.7 Dt F_{tu}/3.0$

 $V_a = 2.7(0.164")(0.033")(45,000 psi)/3.0$

 $V_a = 219 \text{ lb.}$



<u>Alternate Installation – Strap Anchor to CMU</u> (Continued)

Bearing of #8 Screw on Frame

$$V_a = F_eDt/K_D = (3,350 \text{ psi})(0.164")(0.75")/(10(0.164)+0.5) = 193 \text{ lb}$$

Capacity of Connection is 135 lb (only one concrete screw per strap)



Alternate Installation - Strap Anchor to Wood - Cap Installation

Two #8 x 1-1/2" Flat Head Screws securing strap to substrate

Spruce-Pine-Fir 2x Wood Substrate Minimum (G=0.42)

Two #8 Screws securing strap to frame

3/4" thick Wood Frame

20 gauge (0.033" thick) 33 KSI Steel Strap Anchor

1/4" Maximum Shim Space

Allowable Withdrawal of #8 x 1-1/2" Flat Head Screw

$$W' = 1.6(82 \text{ lb/inch})(1.5") = 197 \text{ lb}$$

Allowable Pull-over of #8 x 1-1/2" Flat Head Screw

$$P_{\text{nov}}/\Omega = 1.5 \text{td}_{\text{w}} F_{\text{u}}/\Omega = 1.5(0.033")(0.332")(45,000 \text{ psi})/3.0 = 247 \text{ lb}$$

Bearing of #8 Screw on Frame

$$V_a = F_eDt/K_D = (3,350 \text{ psi})(0.164")(0.75")/(10(0.164)+0.5) = 193 \text{ lb}$$

Bearing of #8 Screw on Strap Anchor

 $V_a = 2.7 Dt F_{tu}/3.0$

 $V_a = 2.7(0.164")(0.033")(45,000 psi)/3.0$

 $V_a = 219 \text{ lb.}$

Capacity of Connection is 193 lb

Capacity for Two Screws is 386 lb



72" x 79" XX Door +50/-55 psf

Anchorage Requirements

Through Frame validated by test.

Through Frame Capacity: 110 lb/anchor

Minimum Alternate Anchor Capacity: 122 lb/anchor

Minimum Alternate > As-Tested Alternate Anchorages OK



Appendix A

Revision Log

<u>Identification</u> <u>Date</u> <u>Page & Revision</u>

Original Issue 04/02/23 Not Applicable